## GATE SOLVED PAPER - CE

## 2004

Q. $2 \quad$ Rivet value is defined as
(A ) lesser of the bearing strength of rivet and the shearing strength of the rivet
(B) lesser of the bearing strength of rivet and the tearing strength of thinner plate
(C) greater of the bearing strength of rivet and the shearing of the rivet
(D) lesser of the shearing strength of the rivet and the tearing strength of thinner plate

A square steel slab base of area $1 \mathrm{~m}^{2}$ is provided for a column made of two rolled channel sections. The $300 \mathrm{~mm} \times 300 \mathrm{~mm}$ column carries an axial compressive load of 2000 kN . The line of action of the load passes through the centroid of the column section as well as of the slab base. The permissible bending stress in slab base is 185 M Pa . The required minimum thickness of the slab base is
(A) 110 mm
(B) 89 mm
(C) 63 mm
(D) 55 mm

In a plate girder, the web plate is connected to the flange plates by fillet welding. The size of the fillet welds is designed to safely resist
(A) the bending stresses in the flanges
(B) the vertical shear force at the section
(C) the horizontal shear force between the flanges and the web plate
(D) the forces causing buckling in the web

A strut in a steel truss is composed of two equal angle ISA $150 \mathrm{~mm} \times 150 \mathrm{~mm}$ of thickness 10 mm connected back-to-back to the same side of a gusset plate. T he cross sectional area of each angle is $2921 \mathrm{~mm}^{2}$ and moment of inertia $\left(I_{x x}=I_{y y}\right)$ is $6335000 \mathrm{~mm}^{4}$. The distance of the centroid of the angle from its surface ( $\mathrm{C}_{x}=\mathrm{C}_{y}$ ) is 40.8 mm . The minimum radius of gyration of the strut is
(A) 93.2 mm
(B) 62.7 mm
(C) 46.6 mm
(D) 29.8 mm

Two equal angles ISA $100 \mathrm{~mm} \times 100 \mathrm{~mm}$ of thickness 10 mm are placed back- to-back and connected to the either side of a gusset plate through a single row of 16 mm diameter rivets in double shear. The effective are $775 \mathrm{~mm}^{2}$ and $950 \mathrm{~mm}^{2}$ , respectively. If the angles are NOT tack riveted, the net effective area of this pair of angles is
(A) $3650 \mathrm{~mm}^{2}$
(B) $3450 \mathrm{~mm}^{2}$
(C) $3076 \mathrm{~mm}^{2}$
(D) $2899 \mathrm{~mm}^{2}$

A moment M of magnitude 50 kNm is transmitted to a column flange through a bracket bb using four 20 mm diameter rivets as shown in the figure.


The shear force induced in the rivet $A$ is
(A) 250 kN
(B) 175 kN
(C) 125 kN
(D) 88.4 kN

A propped cantilever of span $L$ is carrying a vertical concentrated load acting at midspan. The plastic moment of the section is $M_{p}$. The magnitude of the collapse load is
(A) $\frac{8 M_{P}}{L}$
(B) $\frac{6 M_{p}}{L}$
(C) $\frac{4 M_{p}}{L}$
(D) $\frac{2 M_{P}}{L}$

Real matrices $[\mathrm{A}]_{3 \times 1},[\mathrm{~B}]_{3 \times 3}[\mathrm{C}]_{3 \times 5}[\mathrm{D}]_{5 \times 5}$ and $[\mathrm{F}]_{5 \times 1}$ are given. M atrices $[\mathrm{B}]$ and [E] are symmetric.
Following statements are made with respect to these matrices.

1. M atrix product $[F]^{\top}[C]^{\top}[B][C][F]$ is a scalar.
2. $M$ atrix product $[D]^{\top}[F][D]$ is always symmetric.

With reference to above statements, which of the following applies?
(A) Statement 1 is true but 2 is false
(B) Statement 1 is false but 2 is true
(C) B oth the statement are true
(D) B oth the statement are false

The summation of series $S=2+\frac{5}{2}+\frac{8}{2^{2}}+\frac{11}{2^{3}}+$
(A) 4.50
(B) 6.0
(C) 6.75
(D) 10.0

The value of the function $f(x)=\lim _{x \rightarrow 0} \frac{x^{3}+x^{2}}{2 x^{3}-7 x^{2}}$ is
(A) 0
(B) $-\frac{1}{7}$
(C) $\frac{1}{7}$
(D) $\infty$

The eigenvalues of the matrix $\left[\begin{array}{cc}4 & -2 \\ -2 & 1\end{array}\right]$
(A) are 1 and 4
(B) are - 1 and 2
(C) are 0 and 5
(D) cannot be determined
Q. 12
Q. 16 Hardness of water is directly measured by titration with ethylene-di-aminetetracetic acid (EDTA) using
(A) eriochrome black T indicator
(B) ferroin indicator
(C) methyl orange indicator
(D) phenolphthalein indicator

The function $f(x)=2 x^{3}-3 x^{2}-36 x+2$ has its maxima at
(A) $x=-2$ only
(B) $x=0$ only
(C) $x=3$ only
(D) both $x=-2$ and $x=3$

Biotransformation of an organic compound having concentration (x) can be modeled using an ordinary differential equation $\frac{d x}{d t}+k x^{2}=0$, where $k$ is the
reaction rate constant, If $x=a$ at $t=0$, the solution of the equation is reaction rate constant, If $x=a$ at $t=0$, the solution of the equation is
(A) $x=a e^{-k t}$
(B) $\frac{1}{\mathrm{x}}=\frac{1}{\mathrm{a}}+\mathrm{kt}$
(C) $x=a\left(1-e^{-k t}\right)$
(D) $x=a+k t$

A hydraulic structure has four gates which operate independently. The probability of failure of each gate is 0.2 . Given that gate 1 has failed, the probability that both gates 2 and 3 will fail is
(A) 0.240
(B) 0.200
(C) 0.040
(D) 0.008

M ost of the turbidity meters work on the scattering principle. The turbidity value so obtained is expressed in
(A) CPU
(B) FTU
(C) JTU
(D) NTU

The organism, which exhibits very nearly the characteristics of an ideal pathogenic indicator is
(A) Entamoeba histolytica
(B) Escherichia coli
(C) Salmonella typhi
(D) Vibrio comma

The present population of a community is 28000 with an average water consumption of $4200 \mathrm{~m}^{3} / \mathrm{d}$. The existing water treatment plant has a design capacity of 6000 $\mathrm{m}^{3} / \mathrm{d}$. It is expected that the population will increase to 44000 during the next 20 years. The number of years from now when the plant will reach its design capacity, assuming an arithmetic rate of population growth, will be
(A) 5.5 years
(B) 8.6 years
(C) 15.0 years
(D) 16.5 years

Water samples ( X and Y ) from two different source were brought ot the laboratory for the measurement of dissolved oxygen (DO) using modified Winkler method. Samples were transferred to 300 ml BOD bottles. 2 ml of $\mathrm{MnSO}_{4}$ solution and 2 mL of alkaliodide-azide reagent were added to the bottles and mixed. Samples $X$ developed a brown precipitate, whereas sample $Y$ developed a white precipitate. In reference to these observations, the correct statement is
(A) both the samples were devoid of DO
(B) sample $X$ was devoid of DO while sample $Y$ contained DO
(C) sample $X$ contained DO while sample $Y$ was devoid of DO
(D) both the samples contained DO

A standard multiple-tube fermentation test was conducted on a sample of water from a surface stream. The results of the analysis for the confirmed test are given below.

| Simple size (mL) | No. of positive <br> results out of 5 <br> tubes | No. of negative <br> results out of 5 <br> tubes |
| :---: | :---: | :---: |
| 1.0 | 4 | 1 |
| 0.1 | 3 | 2 |
| 0.01 | 1 | 4 |

M PN index and 95\% confidence limits for combination of positive results when the tubes used per dilutions ( $10 \mathrm{~mL}, 1.0 \mathrm{~mL}, 0.1 \mathrm{~mL}$ )

| Combination of positive | M PN index per 100 mL | 95\% confidence limit |  |
| :---: | :---: | :---: | :---: |
|  |  | Lower | upper |
| 4-2-1 | 26 | 12 | 65 |
| 4-3-1 | 33 | 15 | 77 |

Using the above MPN index table, the Most Probable Number MPN of the sample is
(A) 26
(B) 33
(C) 260
(D) 330

The design parameter for flocculation is given by a dimensionless number Gt , where G is the velocity gradient and t is the detention time. Values of Gt ranging from $10^{4}$ to $10^{5}$ are commonly used, with $t$ ranging from 10 to 30 min . The most preferred combination of G and t to produce smaller and denser flocs is
(A) large $G$ values with short $t$
(B) Iarge $G$ values with long $t$
(C) smaller $G$ values with short $t$
(D) small $G$ values with short $t$

Chlorine gas used for disinfection combines with water to form hypochlorous acid $(\mathrm{HOCl})$. The HOCl ionizes to form hypochlorite ( $\mathrm{OCL}^{-}$) in a reversible reaction : $\mathrm{HOCl} \Leftrightarrow \mathrm{H}^{+}+\mathrm{OCl}^{-}\left(\mathrm{k}=2.7 \times 10^{-8}\right.$ at $\left.20^{\circ} \mathrm{C}\right)$, the equilibrium of which is government by pH . The sum of HOCl and $\mathrm{OCL}^{-}$is known as free chlorine residual and HOCl in the free chlorine residual is available at a pH value
(A) 4.8
(B) 6.6
(C) 7.5
(D) 9.4

An analysis for determination of solids in the return sludge of Activated Sludge Process was done as follows:

1. A crocible was dried to a constant mass of 62.485 g .
2. 75 mL of a well-mixed sample was taken in the 62.485 g .
3. The crucible with the sample was dried to a constant mass of 65.020 g in a drying oven at $104^{\circ} \mathrm{C}$.
4. The crucible with the dried sample was placed in a muffle furnace at $600^{\circ} \mathrm{C}$ , for an hour. A fter cooling, the mass of the crucible with residues was 63145 g.
The concentration of organic fraction of solids present in the return sludge sample is
(A) $8800 \mathrm{mg} / \mathrm{L}$
(B) $25000 \mathrm{mg} / \mathrm{L}$
(C) $33800 \mathrm{mg} / \mathrm{L}$
(D) $42600 \mathrm{mg} / \mathrm{L}$

A portion of waste water sample was subjected to standard BOD test (5 days, $20^{\circ} \mathrm{C}$ ), yielding a value of $180 \mathrm{mg} / \mathrm{L}$. The reaction rate constant (to the base'e) at $20^{\circ} \mathrm{C}$ was taken as 0.18 per day. The reaction rate other temperature may be estimated by $k_{T}=k_{20}(1.047)^{\top}-20$. The temperature at which the other portion of the sample shoudl be tested, to exert the same BOD in 2.5 days, is
(A) $4.9^{\circ} \mathrm{C}$
(B) $24.9^{\circ} \mathrm{C}$
(C) $31.7^{\circ} \mathrm{C}$
(D) $35.0^{\circ} \mathrm{C}$

The following data are given for a channel-type grit chamber of length 7.5 m .

1. flow-through velocity $=0.3 \mathrm{~m} / \mathrm{s}$
2. the depth of waste water at peak flow in the channel $=0.9 \mathrm{~m}$
3. specific gravity of inorganic particles $=2.5$
4. $g=9.80 \mathrm{~m} / \mathrm{s}^{2}, \mu=1.002 \times 10^{-3} \mathrm{~N}-\mathrm{s} / \mathrm{m}^{2}$ at $20^{\circ} \mathrm{C}$,

$$
\rho_{\mathrm{w}}=1000 \mathrm{~kg} / \mathrm{m}^{3}
$$

Assuming that the Stoke's law is valid, the largest diameter particles that would be removed with 100 per cent efficiency is
(A) 0.04 mm
(B) 0.21 mm
(C) 1.92 mm
(D) 6.64 mm

An existing 300 mm diameter circular sewer is laid at a slope of $1: 28$ and carries a peak discharge of $1728 \mathrm{~m}^{3} / \mathrm{d}$. Use the partial flow diagram shown in the given figure and assume $M$ anning's $n=0.015$


Partial flow diagram for circular sewer
At the peak discharge, the depth of flow and the velocity are, respectively,
(A) 45 mm and $0.28 \mathrm{~m} / \mathrm{s}$
(B) 120 mm and $0.50 \mathrm{~m} / \mathrm{s}$
(C) 150 mm and $0.57 \mathrm{~m} / \mathrm{s}$
(D) 300 mm and $0.71 \mathrm{~m} / \mathrm{s}$

The $X$ component of velocity in a two dimensional incompressible flow is given by $u=1.5 x$. At the point $(x, y)=(1,0)$, the $y$ component of velocity $v=0$. The equation for the $y$ component of velocity is
(A) $v=0$
(B) $v=1.5 y$
(C) $v=-1.5 x$
(D) $v=-1.5 y$

A frictionless fluid of density $\rho$ flow through a bent pipe as shown below. If $A$ is the cross sectional area and $V$ is the velocity of flow, the forces exerted on segment 1-2 of the pipe in the $x$ and $y$ direction are, respectively,

(A) $\rho A V^{2} ; 0$
(B) $\rho A V^{2} ; \sqrt{2} \rho A V^{2}$
(C) $0 ; 0$
(D) $0 ; \frac{1}{\sqrt{2}} \rho A V^{2}$

In the inclined manometer shown in the figure below, the reservoir is large. Its surface may be assumed to remain at a fixed elevation. A is connected to a gas pipeline and the deflection noted on the inclined glass tube is 100 mm A ssuming $\theta=30^{\circ}$ and the manometric fluid as oil with specific gravity of 0.86 , the pressure at $A$ is

(A) 43 mm water (vaccum)
(B) 43 mm water
(C) 86 mm water
(D) 100 mm water

For a pipe of radius, $r$, flowing half full under the action of gravity, the hydraulic depth is
(A) $r$
(B) $\frac{\pi r}{4}$
(C) $\frac{r}{2}$
(D) $0.379 r$

A wide channel is 1 m deep and has a velocity of flow, V , as $2.13 \mathrm{~m} / \mathrm{s}$. If a disturbance is caused, an elementary wave can travel upstream with a velocity of
(A) $1.00 \mathrm{~m} / \mathrm{s}$
(B) $2.13 \mathrm{~m} / \mathrm{s}$
(C) $3.13 \mathrm{~m} / \mathrm{s}$
(D) $5.26 \mathrm{~m} / \mathrm{s}$

An aircraft is flying in level flight at a speed of $200 \mathrm{~km} / \mathrm{hr}$ through air (density, $\rho=1.2 \mathrm{~kg} / \mathrm{m}^{3}$, and viscosity $\mu=1.6 \times 10^{-5} \mathrm{Ns} / \mathrm{m}^{2}$ ). The lift coefficient at this speed is 0.4 and the drag coefficient is 0.0065 . The mass of the aircraft is 800 kg . The effective lift area of the aircraft is
(A) $21.2 \mathrm{~m}^{2}$
(B) $10.6 \mathrm{~m}^{2}$
(C) $2.2 \mathrm{~m}^{2}$
(D) $1.1 \mathrm{~m}^{2}$

A velocity field is given as $V=2 y \hat{i}+3 x \hat{j}$ where $x$ and $y$ are in metres. The acceleration of a fluid particle at $(x, y)=(1,1)$ in the $x$ direction is
(A) 0
(B) $5.00 \mathrm{~m} / \mathrm{s}^{2}$
(C) $6.00 \mathrm{~m} / \mathrm{s}^{2}$
(D) $8.48 \mathrm{~m} / \mathrm{s}^{2}$

The velocity in $\mathrm{m} / \mathrm{s}$ at a point in a two-dimensional flow is given as $\overline{\mathrm{V}}=2 \mathrm{y} \hat{\mathrm{i}}+3 x \hat{\mathrm{j}}$ . The equation of the stream the passing through the point is
(A ) $3 \mathrm{~d} x-2 d y=0$
(B) $2 x+3 y=0$
(C) $3 d x+2 d y=0$
(D) $x y=6$

A fire protection system is supplied from a water tower with a bent pipe as shown in the figure. The pipe friction $f$ is 0.03 . Ignoring all minor losses, the maximum discharge, Q , in the pipe is

(A) $31.7 \mathrm{lit} / \mathrm{sec}$
(B) $24.0 \mathrm{lit} / \mathrm{sec}$
(C) 15.9 lit/ sec
(D) $12.0 \mathrm{lit} / \mathrm{sec}$

A steady flow occurs in an open channel with lateral inflow of $\mathrm{qm}^{3} \mathrm{~s}$ per unit width as shown in the figure. The mass conservation equation is

(A) $\frac{\partial q}{\partial x}=0$
(B) $\frac{\partial Q}{\partial x}=0$
(C) $\frac{\partial \mathrm{Q}}{\partial \mathrm{x}}-\mathrm{q}=0$
(D) $\frac{\partial \mathrm{Q}}{\partial \mathrm{x}}+\mathrm{q}=0$

A steep wide rectangular channel takes off from a reservoir having an elevation of 101.2 m . At the entrance, the bottom elevation of the channel is 100 m . If the slope of the channel is increased by $4 \%$, the discharge per unit length in the channel will be
(A) $2.24 \mathrm{~m}^{2} / \mathrm{s}$
(B) higher than $2.24 \mathrm{~m}^{2} / \mathrm{s}$ by $4 \%$
(C) higher than $2.24 \mathrm{~m}^{2} / \mathrm{s}$ by $2 \%$
(D) choked

The height of a hydraulic jump in the stilling pool of $1: 25$ scale model was observed to be 10 cm . The corresponding prototype height of the jump is
(A) not determinable from the data given
(C) 0.5 m
(D) 0.1 m

A thin flat plate $0.5 \mathrm{~m} \times 0.7 \mathrm{~m}$ in size settles in a large tank of water with a terminal velocity of $0.12 \mathrm{~m} / \mathrm{s}$. The coefficients of drag $C_{D}=\frac{1.328}{\sqrt{R_{L}}}$ for a laminar boundary and $C_{D}=\frac{0.072}{\left(R_{L}\right)^{1 / 5}}$ for a turbulent boundary layer, where $R_{L}$ is the plate Reynolds number. A Ssume $\mu=10^{-3} \mathrm{Ns} / \mathrm{m}^{2}$ and $\rho=1000 \mathrm{~kg} / \mathrm{m}^{3}$


The submerged weight of the plate is
(A) 0.115 N
(B) 0.0118 N
(C) 0.0231 N
(D) 0.0376 N

The allowable Net Positive Sustion Head (NPSH) for a pump provided by the manufacturer for a flow of $0.05 \mathrm{~m}^{3} / \mathrm{s}$ is 3.3 m . The temperature of water is $30^{\circ} \mathrm{C}$ (vapour pressure head absolute $=0.44 \mathrm{~m}$ ), atmosphere pressure is 200 kPa absolute and the heat loss from the reservoir to pump $0.3 \mathrm{Nm} / \mathrm{N}$. The maximum height of the pump above the sustion reservoir is
(A) 10.19 m
(B) 6.89 m
(C) 6.15 m
(D) 2.86 m

## Common Data For Questions. 41 and 42

The laminar flow takes place between closely spaced parallel plates as shown in figure below. The velocity profile is given by $u=V \frac{y}{h}$. The gap height, $h$, is 5 mm and the space is filled with oil (specific gravity $=0.86$, viscosity $\mu=2 \times 10^{-4} \mathrm{Ns} / \mathrm{m}^{2}$ ). The bottom plate is stationery and the top plate moves with a steady velocity of $\mathrm{V}=5 \mathrm{~cm} / \mathrm{s}$. The area of the plate is $0.25 \mathrm{~m}^{2}$

Q. 41 The ratio of rotation of a fluid particle is given by
(A) $\omega_{y}=0 ; \omega_{2}=-\frac{y}{2 h}$
(B) $\omega_{y}=0 ; \omega_{z}=-\frac{y}{h}$
(C) $\omega_{y}=\frac{y}{h} ; \omega_{z}=\frac{y}{h}$
(D) $\omega_{y}=\frac{y}{h} ; \omega_{2}=0$

The power required to keep the plate in steady motion is
(A) $5 \times 10^{-4}$ watts
(B) $10^{-5}$ watts
(C) $2.5 \times 10^{-5}$ watts
(D) $5 \times 10^{-5}$ watts

A 10 m thick clay layer is underlain by a sand layer of 20 m depth (see figure below). The water table is 5 m below the surface of clay layer. The soil above the water table is capillary saturated. The value of $\gamma_{\text {sat }}$ is $19 \mathrm{kN} / \mathrm{m}^{3}$. The unit weight of water is $\gamma_{w}$. If now the water table rises to the surface, the effective stress at at point $P$ on the interface will

(A ) increase by $5 \gamma_{\text {w }}$
(B) remain unchanged
(C) decrease by $5 \gamma_{\mathrm{w}}$
(D) decrease by $10 \gamma_{\text {w }}$

A unit volume of a mass of saturated soil is subjected to horizontal seepage. The saturated unit weight is $22 \mathrm{kN} / \mathrm{m}^{3}$ and the hydraulic gradient is 0.3 . The resultant body force on the soil mass is
(A ) 1.98 kN
(B) 6.6 kN
(C) 11.49 kN
(D) 22.97 kN

In an undrained triaxial test on a saturated clay, the Poisson's ratio is
(A) $\frac{\sigma_{3}}{\sigma_{1}+\sigma_{3}}$
(B) $\frac{\sigma_{3}}{\sigma_{1}-\sigma_{3}}$
(C) $\frac{\sigma_{1}-\sigma_{3}}{\sigma_{3}}$
(D) $\frac{\sigma_{1}+\sigma_{3}}{\sigma_{3}}$

The undrained cohesion of a remoulded clay soil is $10 \mathrm{kN} / \mathrm{m}^{2}$. If the sensitivity of the clay is 20, the corresponding remoulded compressive strength is
(A) $5 \mathrm{kN} / \mathrm{m}^{2}$
(B) $10 \mathrm{kN} / \mathrm{m}^{2}$
(C) $20 \mathrm{kN} / \mathrm{m}^{2}$
(D) $200 \mathrm{kN} / \mathrm{m}^{2}$

Two circular footings of diameters $D_{1}$ and $D_{2}$ are resting on the surface of the same purely cohesive soil. The ratio of their gross ultimate bearing capacities is
(A) $\frac{D_{1}}{D_{2}}$
(B) 1.0
(C) $\left(\frac{D_{1}}{D_{2}}\right)^{2}$
(D) $\frac{D_{2}}{D_{1}}$

The ratio of saturated unit weight of dry unit weight of soil is 1.25 . If the specific gravity of solids $\left(\mathrm{G}_{s}\right)$ is 2.65 , the void ratio of the soil is
(A) 0.625
(B) 0.663
(C) 0.944
(D) 1.325

The figure given below represents the contact pressure distribution under beneath a

(A) rigid footing on saturated clay
(B) rigid footing on sand
(C) flexible footing on saturated clay
(D) flexible footing on sand

A 6 m thick clay layer undergoes $90 \%$ consolidation four times faster under twoway drainage as compared to one-way drainage. In an identical clay layer of 15 m thickness, two-way drainage will be faster as compared to oneway drainage by
(A) 8 times
(B) 4 times
(C) 2.5 times
(D) 2 times

The figure below shows two flow lines for seepage across an interface between two soil media or different coefficients of permeability. If entrance angle $\alpha_{1}=30^{\circ}$, the exit angle $\alpha_{2}$ will be

(A) $7.50^{\circ}$
(B) $14.03^{\circ}$
(C) $66.59^{\circ}$
(D) $75.96^{\circ}$

An unsupported excavation is made to the maximum possible depth in a clay soil having $\gamma_{\mathrm{t}}=18 \mathrm{kN} / \mathrm{m}^{3}, \mathrm{c}=100 \mathrm{kN} / \mathrm{m}^{2}, \phi=30^{\circ}$. The active earth pressure, according to Rankine's theory, at the base level of the excavation is
(A) $115.47 \mathrm{kN} / \mathrm{m}^{2}$
(B) $54.36 \mathrm{kN} / \mathrm{m}^{2}$
(C) $27.18 \mathrm{kN} / \mathrm{m}^{2}$
(D) $13.0 \mathrm{kN} / \mathrm{m}^{2}$

A retaining wall of height 8 m retains dry sand. In the initial state, the soil is loose and has a void ratio of $0.5, \gamma_{d}=35^{\circ} \mathrm{kN} / \mathrm{m}^{3}$ and $\phi=30^{\circ}$. Subsequently, the backfill is compacted to a state where void ratio is $0.4, \gamma_{d}=18.8 \mathrm{kN} / \mathrm{m}^{3}$ and $\phi=35^{\circ}$. The ratio of initial passive thrust to the final passive thrust, according to Rankine's earth pressure theory, is
(A) 0.38
(B) 0.64
(C) 0.77
(D) 1.55

## Common Data For Questions. 57 and 58 :

A group of 16 piles of 10 m length and 0.5 m diameter is installed in a 10 m thick stiff clay layer underlain by rock. The pile-soil adhesion factor is 0.4 ; average shear strength of soil on the sides is 100 kPa ; undrained shear strength of the soil at the base is also 100 kPa .
Q. 57 The base resistance of a single pile is
(A) 40.00 kN
(B) 88.35 kN
(C) 100.00 kN
(D) 176.71 kN
Q. 58 Assuming $100 \%$ efficiency, the group side resistance is
(A) 5026.5 kN
(B) 10000.0 kN
(C) 10053.1 kN
(D) 20106.0 kN

An infinite soil slope with an inclination of $35^{\circ}$ is subjected to seepage parallel to its surface. The soil has $\mathrm{C}^{\prime}=100 \mathrm{kN} / \mathrm{m}^{2}$ and $\phi=35^{\circ}$. Using the concept of mobilized cohesion and friction, at a factor of safety of 1.5 with respect of shear strength, the mobilized friction angle is
(A) $20.02^{\circ}$
(B) $21.05^{\circ}$
(C) $23.33^{\circ}$
(D) $30.00^{\circ}$

Using $\phi_{u}=0$ analysis and assuming planar failure as shown, the minimum factor of safety against shear failure of a vertical of height 4 m in a pure clay having $\mathrm{c}_{\mathrm{u}}=120 \mathrm{kN} / \mathrm{m}^{2}$ and $\gamma_{\text {sat }}=20 \mathrm{kN} / \mathrm{m}^{3}$ is

(A) 1
(B) 6
(C) 10
(D) 20

In the context of collecting undisturbed soil samples of high quality using a spoon sampler, following statements are made:

1. A rea ratio should be less than $10 \%$
2. Clearance ratio should be less than $1 \%$

With reference to above statements, which of the following applies ?
(A) B oth the statements are true
(B) Statements 2 is true but 1 is false
(C) Statement 1 is true but 2 is false
(D) B oth the statements are false

In the context of flexible pavement design, the ratio of contact pressure tyre pressure is called the Rigidity Factor. This factor is less than unity when the tyre pressure is
(A) less than $0.56 \mathrm{~N} / \mathrm{mm}^{2}$
(B) equal to $0.56 \mathrm{~N} / \mathrm{mm}^{2}$
(C) equal to $0.7 \mathrm{~N} / \mathrm{mm}^{2}$
(D) more than $0.7 \mathrm{~N} / \mathrm{mm}^{2}$

The star and Grid pattern of road network was adopted in
(A) Nagpur Road Plan
(B) Lucknow Road Plan
(C) B ombay Road Plan
(D) Delhi Road Plan

The road geometrics in India are designed for the
(A) $98^{\text {th }}$ highest hourly traffic volume
(B) $85^{\text {th }}$ highest hourly traffic volume
(C) $50^{\text {th }}$ highest hourly traffic volume
(D) $30^{\text {th }}$ highest hourly traffic volume

The data given below pertain to the design of a flexible pavement

$$
\text { Initial traffic = } 1213 \text { cvpd }
$$

Traffic growth rate $=8 \%$ per annum
Design life $=12$ years
Vehicle damage factor $=2.5$
Distribution factor $=1.0$
The design traffic in terms of million standard axles (msa) to be catered would be
(A) 0.06 msa
(B) 8.40 msa
(C) 21.00 msa
(D) 32.26 msa

For a road with camber of $3 \%$ and the design speed of $80 \mathrm{~km} / \mathrm{hr}$, the minimum radius of the curve beyond which NO superelevation is needed is
(A) 1680 m
(B) 948 m
(C) 406 m
(D) 280 m

The co-efficient of friction in the longitudinal direction of a highway is estimated as 0.396 . The braking distance for a car moving at a speed of $65 \mathrm{~km} / \mathrm{hr}$ is
(A) 87 m
(B) 45 m
(C) 42 m
(D) 40 m

Three new roads $P, Q$ and $R$ are planned in a district. The data for these roads are given in the table below.

| Road | Length (km) | Number of villages with population |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | less than <br> 2000 | $2000-5000$ | more than <br> 5000 |  |
| P | 20 | 8 | 6 | 1 |
| Q | 28 | 19 | 8 | 4 |
| R | 12 | 7 | 5 | 2 |

Based on the principle of maximum utility, the order of priority for these three roads should be
(A) P, Q, R
(B) $Q, R, P$
(C) $R, P, Q$
(D) $R, Q, P$
Q. 67 A sprinkler irrigation system is suitable when
(A) the land gradient is steep and the steep and the soil is easily erodible
(B) the soil is having low permeability
(C) the water table is low
(D) the crops to be grown have deep roots

The average rainfall for a 3 hour duration storm is 2.7 cm and the loss rate is 0.3 $\mathrm{cm} / \mathrm{hr}$. The flood hydrograph has a base flow of $20 \mathrm{~m}^{3} / \mathrm{s}$ and produces a peak flow of $210 \mathrm{~m}^{3} / \mathrm{s}$. The peak of a 3-h unity hydrograph is
(A) $125.50 \mathrm{~m}^{3} / \mathrm{s}$
(B) $105.50 \mathrm{~m}^{3} / \mathrm{s}$
(C) $77.77 \mathrm{~m}^{3} / \mathrm{s}$
(D) $70.37 \mathrm{~m}^{3} / \mathrm{s}$

The rainfall during three successive 2 hour periods are $0.5,2.8$ and 1.6 cm . The surface runoff resulting from this storm is 3.2 cm . The $\phi$ index value of this storm is
(A) $0.20 \mathrm{~cm} / \mathrm{hr}$
(B) $0.28 \mathrm{~cm} / \mathrm{hr}$
(C) $0.30 \mathrm{~cm} / \mathrm{hr}$
(D) $0.80 \mathrm{~cm} / \mathrm{hr}$

A canal irrigates a portion of a culturable command area of grow sugarcane and wheat. The average discharges required to grow sugarcane and wheat are, respectively, 0.36 and 0.27 cumecs. The time factor is 0.9 . The required design capacity of the canal is
(A) 0.36 cumecs
(B) 0.40 cumecs
(C) 0.63 cumecs
(D) 0.70 cumecs
Q. 71 In the limit state design method of concrete structures, the recommended partial material safety factor $\left(\gamma_{m}\right)$ for steel according to IS:456-2000 is
(A) 1.5
(B) 1.15
(C) 1.00
(D) 0.87
Q. 72 For avoiding the limit state of collapse, the safety of RC structures is checked for appropriate combinations of Dead Load (DL), Imposed Load or Live Load (IL), W ind Load (WL) and Earthquake Load (EL). W hich of the following load combinations is NOT considered ?
(A) $0.9 \mathrm{DL}+1.5 \mathrm{WL}$
(B) $1.5 \mathrm{DL}+1.5 \mathrm{WL}$
(C) $1.5 \mathrm{DL}+1.5 \mathrm{WL}+1.5 \mathrm{EL}$
(D) $1.2 \mathrm{DL}+1.2 \mathrm{IL}+1.2 \mathrm{WL}$

## Common Data For Questions. 76 and 77

At the limit state of collapse, an RC beam is subjected to flexural moment 200 kN m , shear force 20 kN and torque 9 kNm . The beam is 300 mm wide and has a gross depth of 425 mm , with an effective cover of 25 mm . The equivalent nominal shear stress ( $\tau_{\mathrm{ve}}$ ) as calculated by using the design code turns out to be lesser than the design shear strength ( $\tau_{c}$ ) of the concrete.
An RC short column with $300 \mathrm{~mm} \times 300 \mathrm{~mm}$ square cross-section is made of M 20 grade concrete and has 4 numbers, 20 mm diameter longitudinal bars of Fe 415 steel. It is under the action of a concentric axial compressive load. Ignoring the reduction in the area of concrete due to steel bars, the ultimate axial load carrying capacity of the column is
(A) 1659 kN
(B) 1548 kN
(C) 1198 kN
(D) 1069 kN

A simply supported prestressed concreted beam is 6 m long and 300 mm wide. Its goes depth is 600 mm It is prestressed by horizontal cable tendons at a uniform eccentricity of 100 mm . The prestressing tensile force in the cable tendons is 1000 kN . Neglect the self weight of beam. The maximum normal compressive stress in the beam at transfer is
(A) zero
(B) $5.55 \mathrm{~N} / \mathrm{mm}^{2}$
(C) $11.11 \mathrm{~N} / \mathrm{mm}^{2}$
(D) $15.68 \mathrm{~N} / \mathrm{mm}^{2}$

An RC square footing of side length 2 m and uniform effective depth 200 mm is provided for a $300 \mathrm{~mm} \times 300 \mathrm{~mm}$ column. The line of action of the vertical compressive load passes through the centroid of the footing as well as the column. If the magnitude of the load is 320 kN , the nominal transverse (one way) shear stress in the footing is
(A) $0.26 \mathrm{~N} / \mathrm{mm}^{2}$
(B) $0.30 \mathrm{~N} / \mathrm{mm}^{2}$
(C) $0.34 \mathrm{~N} / \mathrm{mm}^{2}$
(D) $0.75 \mathrm{~N} / \mathrm{mm}^{2}$

The equivalent shear force $\left(V_{c}\right)$ is
(A) 20 kN
(B) 54 kN
(C) 56 kN
(D) 68 kN

The equivalent flexural moment ( $\mathrm{M}_{\text {eq }}$ ) for designing the longitudinal tension steel is
(A) 187 kNm
(B) 200 kNm
(C) 209 kNm
(D) 213 kNm

For linear elastic systems, the type of displacement function for the strain energy is
(A) Iinear
(B) quadratic
(C) cubic
(D) quartic

In a two dimensional stress analysis, the state of stress at a point is shown below. If $\sigma=120 \mathrm{MPa}$ and $\tau=70 \mathrm{MPa}, \sigma_{\mathrm{x}}$ and $\sigma_{\mathrm{y}}$, are respectively,

(A ) 26.7 M Pa and 172.5 M Pa
(B) 54 M Pa and 128 MPa
(C) 67.5 M Pa and 213.3 M Pa
(D) 16 MPa and 138 MPa

For the linear elastic beam shown in the figure, the flexural rigidity, El is $781250 \mathrm{kNm}{ }^{2}$. When $w=10 \mathrm{kN} / \mathrm{m}$, the vertical reaction $R_{A}$ at $A$ is 50 kN . The value of $R_{A}$ for $w=100 \mathrm{kN} / \mathrm{m}$ is

(A) 500 kN
(B) 425 kN
(C) 250 kN
(D) 75 kN
Q. 81 A homogeneous, simply supported prismatic beam of width $B$, depth $D$ and span $L$ is subjected to a concentrated load of magnitude $P$. The load can be placed anywhere along the span of the beam. The maximum flexural stress developed in beam is
(A) $\frac{2 P L}{3 B D^{2}}$
(B) $\frac{3 \mathrm{PL}}{4 \mathrm{BD}^{2}}$
(C) $\frac{4 \mathrm{PL}}{3 \mathrm{BD}^{2}}$
(D) $\frac{3 \mathrm{PL}}{2 \mathrm{BD}^{2}}$

A circular solid shaft of span $L=5 \mathrm{~m}$ is fixed at one end and free at other end. A twisting moment $\mathrm{T}=100 \mathrm{kNm}$ is applied at the free end. The torsional rigidity GH is $50000 \mathrm{kN} \mathrm{m}^{2} / \mathrm{rad}$.
Following statements are made for this shaft :

1. The maximum rotation is 0.01 rad
2. The torsional strain energy is 1 kNm

With reference to the above statements, which of the following applies?
(A) B oth statements are true
(B) Statement 2 is true but 2 is false
(C) Statement 2 is true but 1 is false
(D) B oth the statements are false

## Common Data For Questions. 83 \& 84 :

A threespan continuous beam has an internal hinge at $B$. Section $B$ is at the mid-span of AC. Section E is at the mid-span of CG. The 20 kN load is applied at section B whereas 10 kN loads are applied at sections D and F as shown in the figure. Span GH is subjected to uniformly distributed load of magnitude $5 \mathrm{kN} / \mathrm{m}$. For the loading shown, shear force immediate to the right of section E is 9.84 kN upwards and the hogging moment at section E is 10.31 kNm


The magnitude of the shear force immediate to the left and immediate to the right of section B are respectively
(A) 0 and 20 kN
(B) 10 kN and 10 kN
(C) 20 kN and 0
(D) 9.84 kN and 10.16 kN

The vertical reaction at support H is
(A) 15 kN upward
(B) 9.84 kN upward
(C) 15 kN downward
(D) 9.84 kN downward

The unit load method used in structural analysis is
(A) applicable only to statically indeterminate structures
(B) another name for stiffness method
(C) an extension of Maxwell's reciprocal theorem
(D) derived from Castigliano's theorem

For a linear structural system, minimization of potential energy yields
(A) compatibility conditions
(B) constitutive relations
(C) equilibrium equations
(D) strain-displacement relations

For the plane truss shown in the fig. the number of zero force members for the given loading is

(A) 4
(B) 8
(C) 11
(D) 13

For the plane frame with an overhang as shown below, assuming negligible axial deformation, the degree of static indeterminacy, $d$, and the degree of kinematic indeterminacy, k, are

(A) $\mathrm{d}=3$ and $\mathrm{k}=10$
(B) $\mathrm{d}=3$ and $\mathrm{k}=13$
(C) $\mathrm{d}=19$ and $\mathrm{k}=10$
(D) $\mathrm{d}=9$ and $\mathrm{k}=13$

The plane frame below is analyzed by neglecting axial deformations. Following statements are made with respect to the analysis :


1. Column $A B$ carries axial force only
2. Vertical deflection at the center of beam $B C$ is 1 mm With reference to the above statements, which of the following applies ?
(A) B oth the statements are true
(B) Statement 1 is true but 2 is false
(C) Statement 2 is true but 1 is false
(D) B oth the statements are false

A three hinged parabolic arch $A B C$ has a span of 20 m and a central rise of 4 m . The arch has hinges at the ends and at the centre. A train of two point loads of 20 kN and $10 \mathrm{kN}, 5 \mathrm{~m}$ apart, crosses this arch from left to right, with 20 kN load leading. The maximum thrust induced at the supports is
(A) 25.00 kN
(B) 28.13 kN
(C) 31.25 kN
(D) 32.81 kN

ANSWER KEY

| $\underline{9004}$ |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 |
| (C) | (A) | (D) | (C) | (C) | (B) | (B) | (A) | (D) | (B) |
| 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 |
| (C) | (A) | (B) | (C) | (C) | (A) | (B) | (C) | (C) | (B) |
| 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 |
| (D) | (B) | (C) | (D) | (B) | (C) | (D) | (C) | (B) | (B) |
| 31 | 32 | 33 | 34 | 35 | 36 | 37 | 38 | 39 | 40 |
| (A) | (D) | (D) | (A) | (B) | (B) | (C) | (B) | (A) | (C) |
| 41 | 42 | 43 | 44 | 45 | 46 | 47 | 48 | 49 | 50 |
| (A) | (C) | (C) | (B) | (B) | (B) | (B) | (B) | (A) | (B) |
| 51 | 52 | 53 | 54 | 55 | 56 | 57 | 58 | 59 | 60 |
| (C) | (A) | (C) | (B) | (B) | (C) | (D) | (C) | (D) | (A) |
| 61 | 62 | 63 | 64 | 65 | 66 | 67 | 68 | 69 | 70 |
| (D) | (C) | (D) | (C) | (D) | (C) | (A) | (B) | (C) | (D) |
| 71 | 72 | 73 | 74 | 75 | 76 | 77 | 78 | 79 | 80 |
| (B) | (C) | (D) | (C) | (A) | (D) | (D) | (A) | (A) | (C) |
| 81 | 82 | 83 | 84 | 85 | 86 | 87 | 88 | 89 | 90 |
| (D) | (B) | (A) | (B) | (A) | (C) | (A) | (D) | (A) | (C) |

